GENERAL NOTES

1.1 Fabrication shall be in accordance with R.G.B. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3". R.G.B. manufacturing procedures are certified by:

Reference	Certification numbers
Houston	R.G.B. #456

1.2 MATERIALS

	ASTM DESIGNATION	MIN. YIELD STRENGTH
Hot Rolled Steel Shapes (W, S, C & L)	A572/A529	Fy = 50 KSI
Hot Rolled Steel Shapes (W)	A992	Fy = 50 KSI
Round Structural Tubing (HSS)	A500	Fy = 42 KSI
Square/Rect. Structural Tubing (HSS)	A500	Fy = 46 KSI
Structural Steel Web Plate	A572/A1011	Fy = 55 KSI
Structural Steel Flange Plates/Bars	A529/A572	Fy = 55 KSI
Cold Formed Light Gage	A653/A1011	Fy = 55 KSI
Roof and Wall Sheets	A792/A653	Fy = 50, 80 KSI
Cable Brace	A475	Extra High Strength
Rod Brace	A36	Fy = 36 KSI
		MIN. TENSILE STRENGTH
Machine Bolts & Nuts	A307	Fu = 60 KSI
High Strength Bolts (1"ø and less)	A325–TYPE 1	Fu = 120 KSI
High Strength Bolts (>1"ø to 1 1/2"ø)	A325–TYPE 1	Fu = 105 KSI
Anchor Bolts (if supplied)	A36/A307/F1554 Gr.36	Fu = 58–80 KSI

1.3 PRIMER
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is R.G.B. Red or Gray Oxide color. This paint is not intended for long term exposure to the elements. R.G.B. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. R.G.B. shall not be responsible for any field applied paint and/or coatings. (Section 6.5 AISC Code of Standard Practice, 14th Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.

1.4 GALVANIZED OR SPECIAL COATINGS:
See Contract Documents

1.5 ALL BOLTS ARE 1/2"ø x 0'–1" A307 (snug–tightened) EXCEPT :
a) Eave strut connection – 1/2"ø x 0'–1 1/4" A307 without washer (unless noted otherwise)
b) Endwall rafter splice – 5/8"ø x 0'–1 3/4" A325–N with washer
c) Endwall column to rafter connection – 1/2"ø x 0'–1 1/4" A325–N without washer
d) Main frame moment splice connections – A325–N with washer, SEE CROSS SECTION for dimensions.
NOTE: One (01) washer is supplied on main frame moment splice and to A325 bolts unless noted otherwise on drawing

1.6 A325 BOLT TIGHTENING REQUIREMENTS
All high strength bolts are A325–N unless specifically noted otherwise. Structural bolts shall be tightened by the turn–of–the–nut or calibrated wrench methods in accordance with the 14th Edition AISC/RCSC "Specification For Structural Joints using ASTM A325 or A490 Bolts". Washers are supplied separately from High Strength Bolts, however, assembly with washers are required before erection. Installation inspection is recommended and be based on Section 9.1 and 9.2 of AISC/RCSC.

Snug–tight is permitted EXCEPT for the following conditions:
a) Building located in high seismic areas; Seismic Design Categories D, E, F
b) Building supporting cranes
c) Building supporting machinery that creates vibration, impact or stress reversal
d) Connections using ASTM A490
e) Connections using slip–critical condition
f) or as prohibited in the contracts/specifications

1.7 CLOSURE STRIPS ARE FURNISHED FOR APPLICATION:
INSIDE– Under roof panels at eave
OUTSIDE – Between endwall panels and rake trim
– Under continuous ridge vent skirts

1.8 ERECTION NOTE:
All bracing, strapping, & bridging shown and provided by R.G.B. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.9 ERECTION AND UNLOADING NOT BY R.G.B.

1.10 SHORTAGES
Any claims or shortages by buyer must be made to R.G.B. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
Claims for correction of alleged misfits will be disallowed unless R.G.B. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of R.G.B.

BUYER/END USE CUSTOMER RESPONSIBILITIES

2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release R.G.B. to fabricate upon receiving such.

2.2 Rigid Global Buildings (hereafter referred to as R.G.B.) standard specifications apply unless stipulated otherwise in the Contract Documents. R.G.B. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.

2.3 In case of discrepancies between R.G.B. structural steel plans and plans for other trades, R.G.B. plans shall govern. (Section. 3 AISC Code of Standard Practices, 14th Edition)

2.4 Approval of R.G.B. drawings and calculations indicates that R.G.B. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the R.G.B. design concepts, assumptions, and loading. (Section 4 AISC Code 14th Edition and MBMA 3.3.3)

2.5 Once the BUYER/END USE CUSTOMER has signed R.G.B. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.

RIGID GLOBAL BUILDINGS

DRAWING PACKAGE

SALES NO.	68375	JOB NO.	147664	BUILDING	A
CUSTOMER	JEFF SHEPARD				
END USER	JEFFREY SHEPARD				
END USE	RESIDENTIAL				
STREET	1684 STONEY CREEK RD				
CITY ST ZIP	BEDFORD, VA 24523				
COUNTY	BEDFORD				

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

Design Code	: IBC 15		
Dead Load (psf)	: Metal building structure only by RGB		
Collateral Load (psf)	: 5		
Wind Load			
Ultimate Design Wind Speed	: Vult (3 sec. gust)	=	115.00 mph
Nominal Design Wind Speed	: Vasd (3 sec. gust)	=	89.08 mph
Risk Category	: II - Normal		
Wind Exposure	: C		
Internal Pressure Coefficient, GCPi	: 0.180 / -0.180		
Design Wind Pressure For Wall	: Based on Nominal Design Wind Speed		
Components Wind Pressure (psf) asd	: 18.1896		
Components Wind Suction (psf) asd	: -20.0010		
Claddings Wind Pressure (psf) asd	: 21.3780		
Claddings Wind Suction (psf) asd	: -23.1894		
Enclosure	: Closed		
Live Load			
Primary Framing (psf)	: 20.00		
Trib. Area Reduction	: No		
Secondary Framing (psf)	: 20.00		
Snow Load			
Ground Snow Load, Pg (psf)	: 40.000		
Roof Snow Load, Pf (psf)	: 30		
Sloped Roof Snow Load, Ps (psf)	: 30		
Snow Exposure Factor, Ce	: 1.000		
Snow Importance Factor, Is	: 1.000		
Thermal Factor, Ct	: 1.000		
Sloped Factor, Cs	: 1.000		
Seismic Load			
Seismic Importance Factor, Ie	: 1.00		
Seismic Occupancy Category	: II - Normal		
Site Class	: D		
Mapped Spectral Response Acceleration	: Ss = 0.170	:S1 =	0.072
Spectral Response Coefficients	: Sds = 0.181	:Sd1 =	0.115
Seismic Design Category	: B		
Basic Force Resisting Systems Used	: Steel System Not Specifically Detailed For Seismic Resistance		
	: Rigid Frames		
	: Braced Frames		
Total Design Base Shear, V (kips)	: Longitudinal = 14.2	Transverse =	14.2
Response Modification Factors, R	: 3.00		
Seismic Response Coefficient, Cs	: 0.061		
Analysis Procedure Used	: Equivalent Lateral Force Procedure		
Rainfall Intensity (in/hr)	: 5.640		
DESIGN AND DETAIL REQUIREMENTS			
Deflection Limit	: Horizontal		
	H/60 = RF & Portal Frame Sidesway		
	L/90 = Girts		
	L/90 = Wall Panel		
	: Vertical		
	L/180 = RF Rafter		
	L/150 = Purlins		
	L/150 = Roof Panel		
	: 1st & 2nd Floor Beam		
	L/360 = Live Load Only		
	L/240 = DL + LL		

BUILDING DESCRIPTION:

Width (ft)	: SEE DRAWINGS
Length (ft)	:
Eave Ht. at BSW (ft)	:
Eave Ht. at FSW (ft)	:
Roof Slope at BSW	:
Roof Slope at FSW	:
Bay Spacing (ft)	: /

COVERING AND TRIMS:

Roof Panels & Trims	
Panel Type	: 26 Ga. PBR
Panel Color	: Ash Gray
Trim Colors	
Eave Trim	: Solar White
Eave Gutter	: Solar White
Gable Trim	: Solar White
Wall Panel & Trims	
Panel Type	: 26 Ga. PBM
Panel Color	: Solar White
Trim Colors	
Corner Trims	: Solar White
Opening Trims	: Solar White
Downspouts	: Solar White
Base Trim	: N/A
Mas. Flash	: N/A
Special Requirements	: NONE

1st & 2nd Floor : DL = 50psf LL = 100psf
Concrete Thickness = 3.5" Light Weight
Req'd Clearance Below Floor = 10ft
Other Building Conditions : Open wall up to 10ft from FFL
: All Columns recessed by 6"

Before erecting your building, please see the Rigid Erection & Safety Manual at rigidbuilding.com/document–library

2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by R.G.B. and R.G.B. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or R.G.B. assumptions will govern (Section 4 and Commentary, AISC Code of Standard Practice, 14th Edition)

2.7 It is the responsibility of the BUYER/END USE CUSTOMER to ensure that R.G.B. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that R.G.B. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by R.G.B.

2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with R.G.B. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (Section 7 AISC Code of Standard Practice, 14th Edition.)

2.9 Rigid Global Buildings is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete, anchor bolt embedment or the adequacy of the anchor bolt in relation to the concrete.
Unless otherwise provided in the Order Documents, R.G.B. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage/anchor bolt embedment and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Chapter IV Section 3.2.2 Metal Building Systems Manual 2012 Edition)

2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to R.G.B. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (Section 7 AISC Code of Standard Practice, 14th Edition)

2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (Section 7 AISC Code of Standard Practice, 14th Edition)

2.12 WARNING In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run–off from copper flashing, wiring, or tubing onto Galvalume should be avoided.

2.13 SAFETY COMMITMENT Rigid Global Buildings has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of R.G.B.
It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees.
Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.

2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.

2.15 It is recommended by Factory Mutual (Reference: B2.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
30	17.90	8.95
40	19.20	9.60
50	20.50	10.25
60	21.80	10.90
70	23.10	11.55
80	24.40	12.20

NOTE:
For Snow/Ice Removal Procedure, Refer to Metal Building System Manual 2012 Edition, Section A9.4, Page A–59

FOR PERMIT

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT RIGID GLOBAL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY RIGID IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN RIGID ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

CUSTOMER:					
JEFF SHEPARD					
SALES NO.:	JOB NO.:	BUILDING:	DWG. NO.:	ISSUE:	
68375	147664	A	C1 OF 2	A	

UNLOADING, HANDLING AND STORING OF MATERIALS

STRUCTURAL

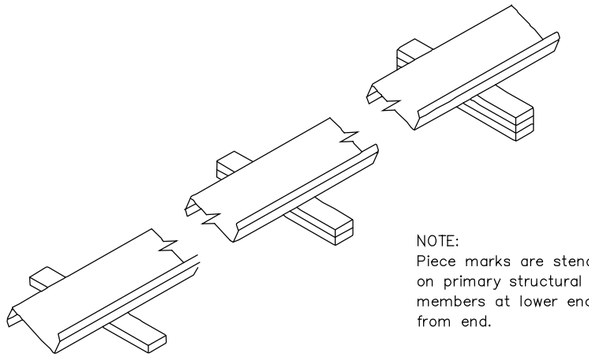
A great amount of time and trouble can be saved if the building site is according to a pre–arranged plan. Proper location and handling of components will eliminate unnecessary handling.

Inspect all shipments prior to releasing the tie–downs for loads that may have shifted during transit, REMEMBER, SAFETY FIRST!

Blocking under the columns and rafters protects the splice plates and the slab from damage during the unloading process. It also facilitates the placing of slings or cables around the members for later lifting and allows members to be bolted together into sub–assemblies while on the ground. Extra care should always be exercised in the unloading operations to prevent injuries from handling the steel and to prevent damage to materials and the concrete slabs.

If water is allowed to remain for extended periods in bundles of primed parts such as girts, purlins etc., the pigment will fade and the paint will gradually soften, reducing the bond to the steel. Therefore, upon receipt of a job, all bundles of primed parts should be stored at an angle to allow any trapped water to drain away and permit air circulation for drying. Puddles of water should not be allowed to collect and remain on columns, rafters or beams for the same reason.

<u>All Primer should be touched up as required before erection!</u>



NOTE:
Piece marks are stenciled on primary structural members at lower end, 1'–0" from end.

WALLS AND ROOF PANELS

RIGID's wall and roof panels including color coated, galvalume and galvanized, provide excellent service under widely varied conditions. All unloading and erection personnel should fully understand that these panels are quality merchandise which merit cautious care in handling.

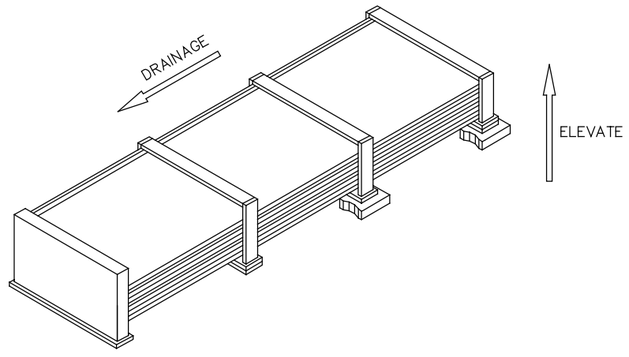
Under no circumstances should panels be handled roughly. Packages of sheets should be lifted off the truck with extreme care taken to insure that no damage occurs to ends of the sheets or to side ribs. The packages should be stored off the ground sufficiently high to allow air circulation underneath the packages. This avoids ground moisture and deters people from walking on the packages. One end of the package should always be elevated to encourage drainage in case of rain.

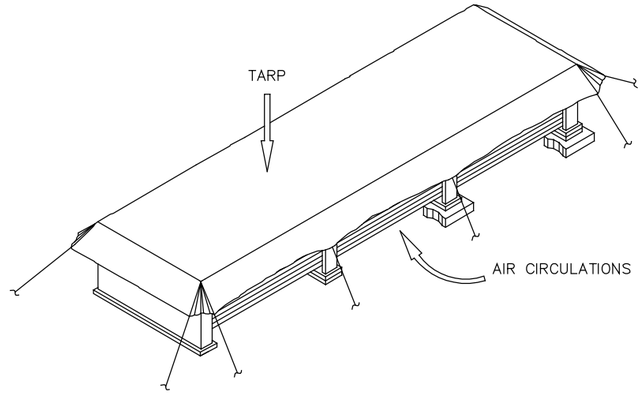
All stacked metal panels are subject, to some degree, to localized discoloration or stain when water is trapped between their closely nested surfaces. RIGID exercises extreme caution during fabricating and shipping operations to insure that all panel stock is kept dry. However, due to climatic conditions, water formed by condensation of humid air can be trapped between stacked sheets. Water can also be trapped between stacked sheets when exposed to rain. This discoloration caused by trapped moisture is often called wet storage stain.

The stain is usually superficial and has little effect on the appearance or service life of the panels as long as it is not permitted to remain on the panels. However, moisture in contact with the surface of the panels over an extended period can severely attack the finish and reduce the effective service life. Therefore, it is imperative that all panels be inspected for moisture upon receipt of the order. If moisture is present, dry the panels at once and store in a dry, warm place.

CAUTION: Care should always be taken when walking on panels. Use saftey lines and nets when necessary! Panels are slippery. Oil or wax applied to the roof and wall panels for protection against weather damage will make them a very slippery surface. Wipe dry any oil that has puddled from bundles stored on a slope. Dew, frost, or other forms of moisture greatly increase the slipperiness of the panels. Always assume panel surface is slippery and act accordingly. Think safety!!

Use wood blocking to elevate and slope the panels in a manner that will allow moisture to drain. Wood blocking placed between bundles will provide additional air circulation. Cover the stacked bundles with a tarp or plastic cover leaving enough opening at the bottom for air to circulate.





When handling or uncrating the panels, <u>lift, rather than slide, them apart.</u> Burred edges may scratch the coated surfaces when sheets are slid over one another. Never allow panels to be walked on while on the ground.

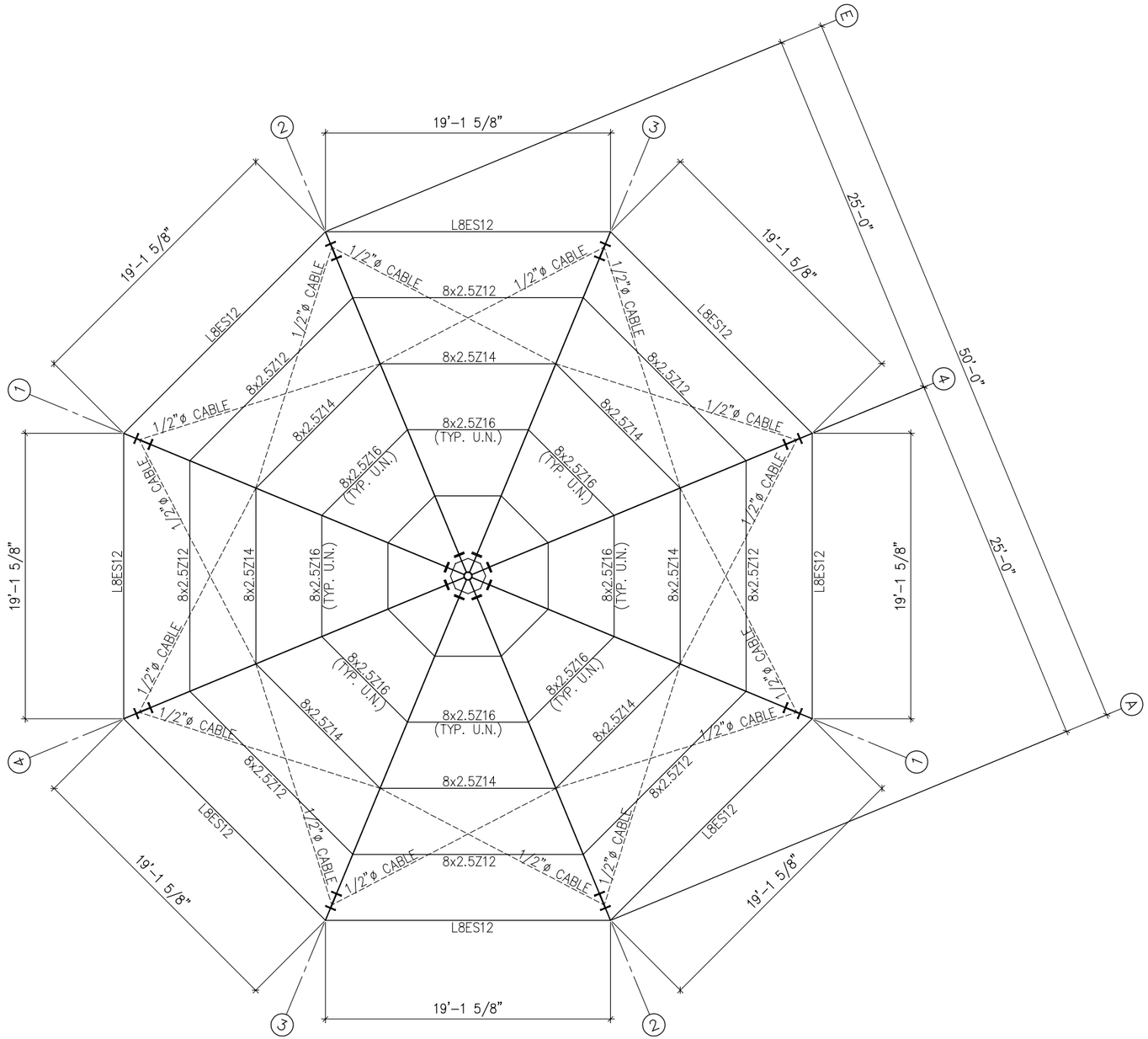
Rough and improper handling of a panel is inexcusable and a prime example of poor job supervision.

NOTE:
Use gloves when handling metal panels to prevent hand injuries. Be aware, of the dangers of handling panels on a windy day. A large panel can catch enough wind to knock a worker off his feet, even at ground level!! <u>Safety first!</u>

<u>GENERAL NOTE:</u>
1. OIL CANNING OF PANELS IS NOT A CAUSE OF REJECTION.
2. EXTREME CARE MUST BE EXERCISED DURING THE ERECTION OF ROOF PANELS AND TRIMS. FOOT TRAFFIC MAY RESULT IN PERMANENT PANEL DISTORTION AND FINISH ABRASION.

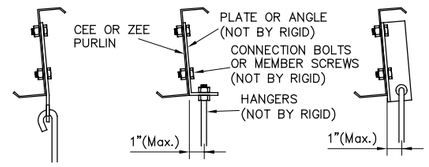
FOR PERMIT

CUSTOMER:				
JEFF SHEPARD				
SALES NO.: 68375	JOB NO.: 147664	BUILDING: A	DWG. NO.: C2 OF 2	ISSUE: A

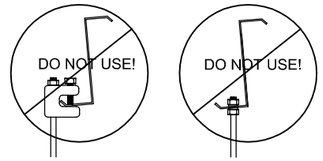


ROOF FRAMING PLAN

NOTE: 6" THK. ROOF AND WALL INSULATION ARE BY OTHERS.
** SEE SHT. 'E-09 TO E-11' FOR CONNECTION DETAILS.

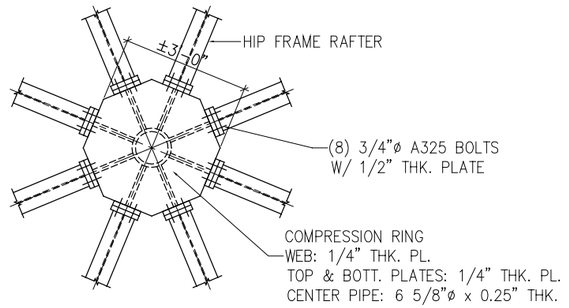


HANGERS ON CEE OR ZEE PURLINS MUST ATTACH TO THE WEB ONLY. HANGERS MUST BE LOCATED SO THAT THE SUPPORTED LOAD DOES NOT EXCEED THAT SPECIFIED IN THE CONTRACT DOCUMENTS. SUGGESTED DETAILS ARE AS SHOWN ABOVE.



DO NOT USE C-CLAMPS OR ANY OTHER HANGER ATTACHED TO THE FLANGE OF THE PURLIN. IT REDUCES THE CAPACITY OF THE PURLIN AND PRODUCE SECONDARY STRESSES.

1 / SF38 TYP. COLLATERAL LOAD CONNECTION DETAIL



COMPRESSION RING DETAIL

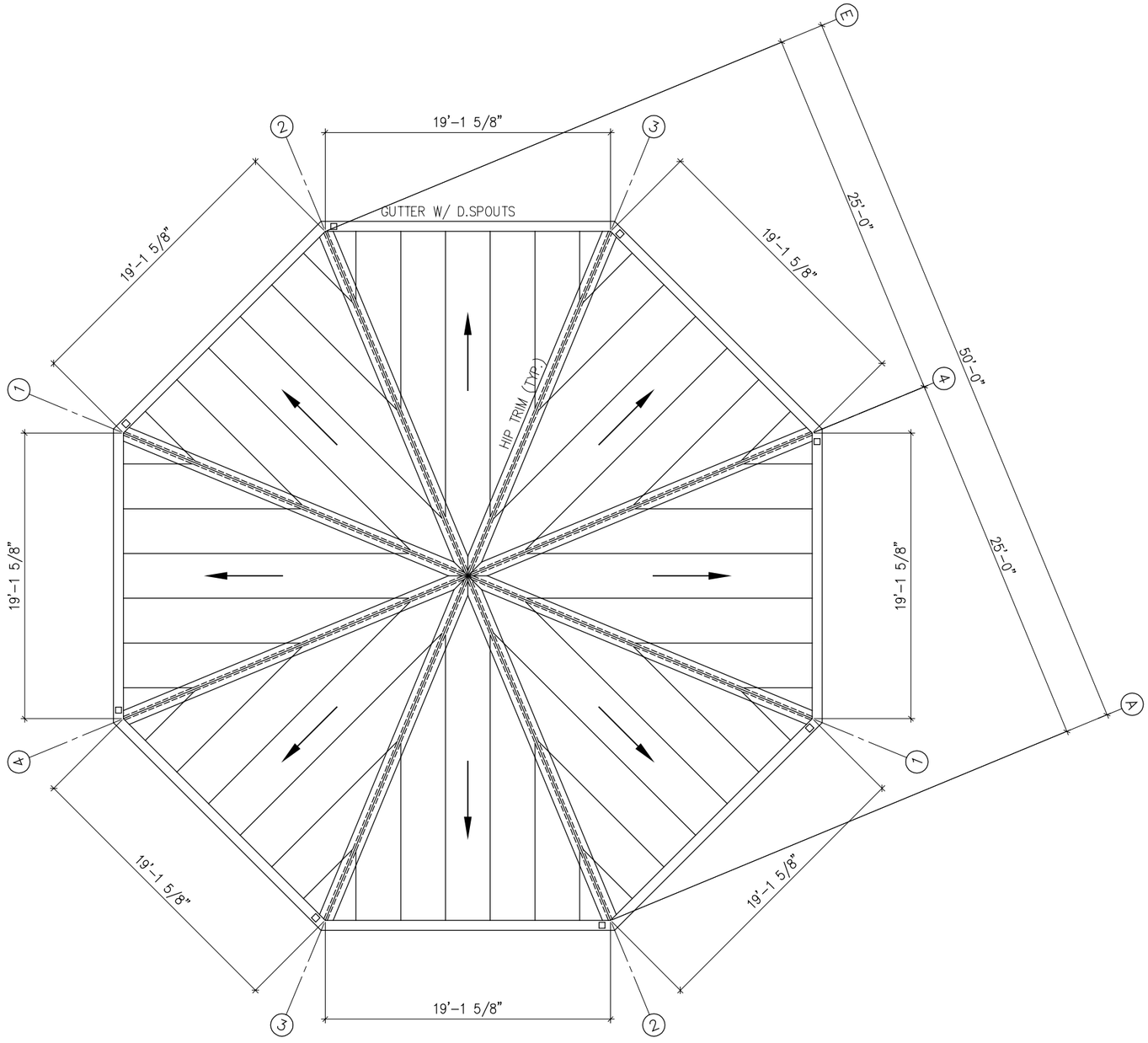
FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

DESCRIPTION	ROOF FRAMING PLAN		
CUSTOMER	JEFF SHEPARD		
END USER	JEFFREY SHEPARD		
END USE	RESIDENTIAL	BUILDING	A
STREET	1684 STONEY CREEK RD		
CITY ST ZIP	BEDFORD, VA 24523		

SALES NO.	JOB NO.	SCALE:	DWG. NO.	ISSUE:
68375	147664	N.T.S.	E01 OF 11	A



ROOF SHEETING PLAN

PANELS: 26 Ga. PBR – Ash Gray

IMPORTANT NOTES:

1. OIL CANNING OF PANELS IS NOT A CAUSE OF REJECTION.
2. EXTREME CARE MUST BE EXERCISED DURING ERECTION OF ROOF PANELS AND TRIMS. FOOT TRAFFIC MAY RESULT IN PERMANENT PANEL DISTORTION AND FINISH ABRASION.

ERECTOR'S NOTE

1. INSTALLER OF STANDING SEAM ROOF PANEL MUST STUDY THE INSTALLATION MANUALS PRIOR TO INSTALLATION. MANUALS ARE PROVIDED WITH THE MATERIALS SHIPMENT BUT CAN BE REQUESTED OR DOWNLOADED FROM THE RIGID GLOBAL BUILDINGS WEBSITE AT www.rigidbuilding.com
2. FAILURE TO INSTALL THE ROOF SHEETS IN ACCORDANCE WITH THE SHEETING DIRECTIONAL ARROWS SHOWN ON THESE PLANS MAY RESULT IN IMPROPER FIT-UP OF THE OUTSIDE CLOSURES (END DAMS) AND POSSIBLY OTHER TRIM COMPONENTS WHICH COULD AFFECT THE OVERALL APPEARANCE AND WEATHER TIGHTNESS OF THE BUILDING. RIGID WILL NOT BE HELD REPONSIBLE FOR THE CHARGES OR ADDITIONAL FIELD WORK DUE TO NOT FOLLOWING SHEETING DIRECTIONAL ARROWS AND OTHER PROCEDURES OUTLINED IN THE ERECTION MANUAL.
3. IN THE EVENT THAT A DISCREPANCY OR ERROR ARISES WITH MATERIALS SHIPPED FOR THIS PROJECT OR ON THESE ERECTION DRAWINGS, THE ERECTOR/INSTALLER MUST NOTIFY RGB PRIOR TO CORRECTING. IF RGB IS NOT NOTIFIED, RGB WILL NOT HONOR BACKCHARGES BY ANY PARTY INVOLVED.
4. MEMBER SCREW AND STITCH SCREW PATTERNS AND LOCATIONS SHALL BE IN ACCORDANCE WITH ROOF AND WALL DETAILS SHOWN ON DWG.# E-10 & E-11.
5. RGB SUPPLIES 5% OVERAGE FOR SCREWS AND ANY CLAIM ON SHORTAGE BECAUSE OF NON-COMPLIANCE WITH THE DRAWINGS SHALL NOT BE RGB'S RESPONSIBLITY.

FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

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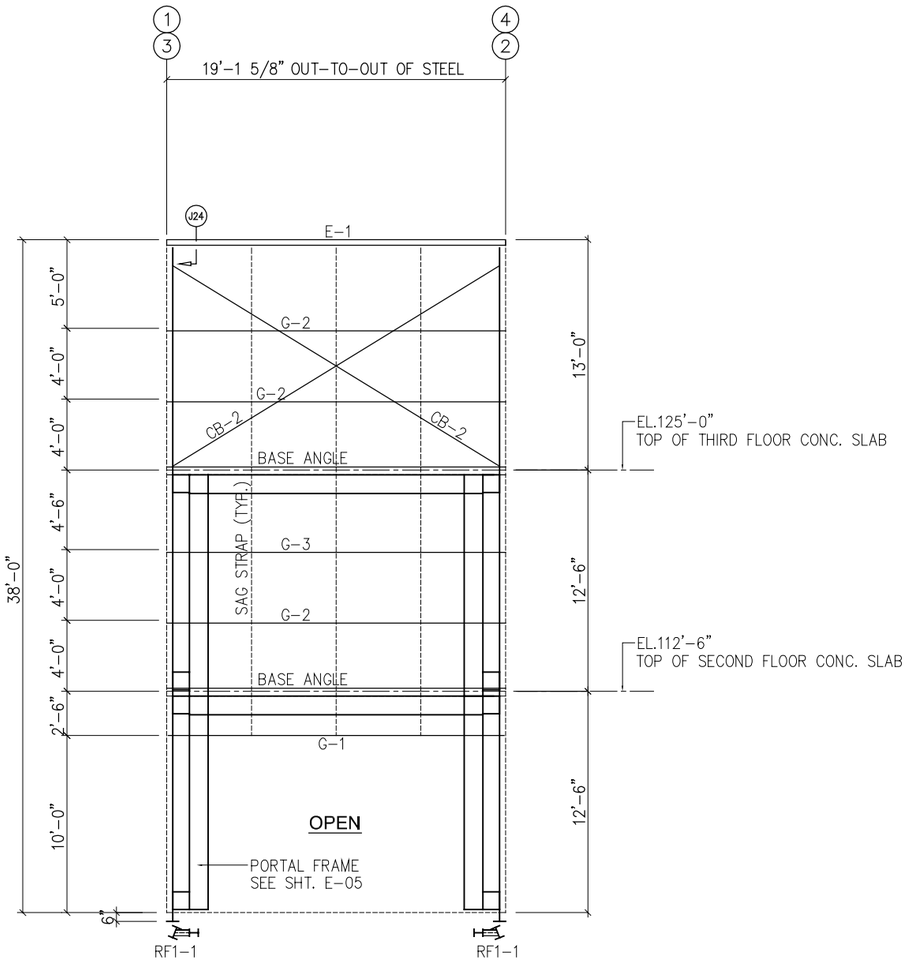
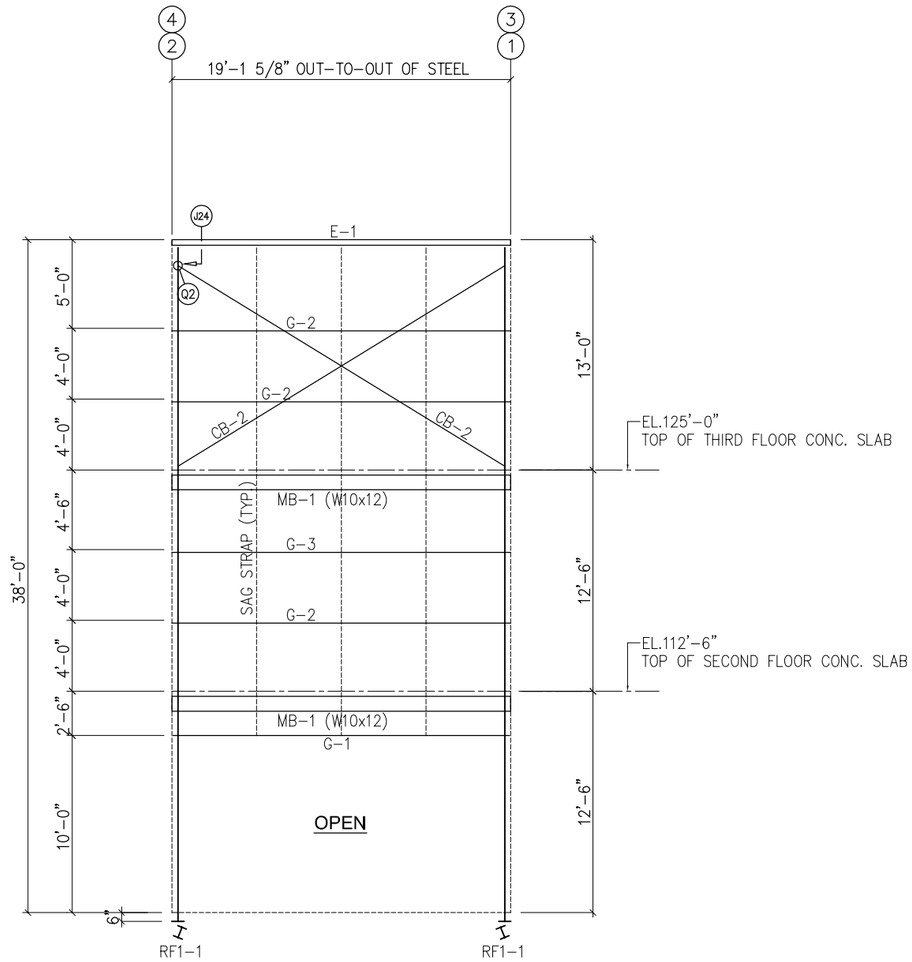
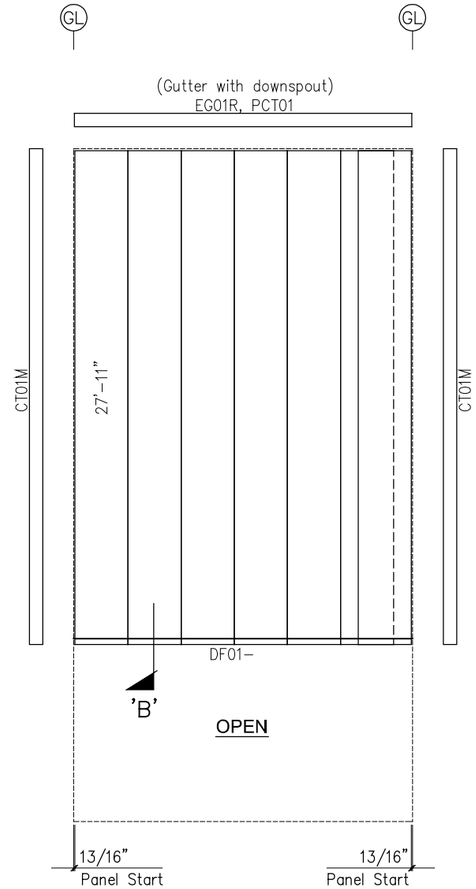
SALES NO.	JOB NO.	SCALE:	DWG. NO.	ISSUE:
68375	147664	N.T.S.	E02 OF 11	A



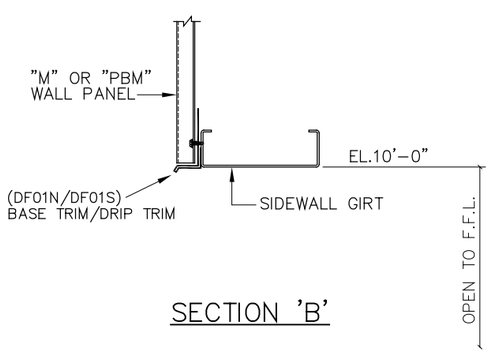
SIDEWALL FRAMING ELEVATION
(4) SIDES TYPICAL W/ PORTAL



SIDEWALL FRAMING ELEVATION
(4) SIDES TYPICAL W/O PORTAL



SIDEWALL SHEETING ELEVATION
(8) SIDES TYPICAL
PANELS: 26 Ga. PBM – SOLAR WHITE



SECTION 'B'

MEMBER TABLE FRAME LINE A	
MARK	PART
E-1	L8ES12
G-1	8x25C16
G-2	8x25Z16
G-3	8x35Z16
CB-2	CB375(2)

FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

DESCRIPTION	SIDEWALL FRAMING AND SHEETING ELEVATIONS		
CUSTOMER	JEFF SHEPARD		
END USER	JEFFREY SHEPARD		
END USE	RESIDENTIAL	BUILDING	A
STREET	1684 STONEY CREEK RD		
CITY ST ZIP	BEDFORD, VA 24523		
SALES NO.: 68375	JOB NO.: 147664	SCALE: N.T.S.	DWG. NO.: E03 OF 11 ISSUE: A

1ST FLOOR TO 2ND FLOOR

SPLICE PLATE & BOLT TABLE

Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	0.750	2.75	6"	3/4"	1'-8 5/16"
SP-2	4	4	0	A325	0.750	2.50	6"	5/8"	1'-10 1/4"

STIFFENER TABLE

Mark	Stiff Mark	Plate Size Width	Thick	Length
RF1-1	St- 1	2.500	0.250	11.50
RF1-3	St- 1	2.500	0.250	17.50

CAP PLATE BOLTS

Mark	Qty	Type	Dia	Length
RF1-5	4	A325	0.750	2.00

BASE PLATE TABLE

Col Mark	Plate Size Width	Thick	Length
BP-1	8"	1/2"	1'-2"
BP-2	1'-0"	5/8"	1'-0"

FBXXA(1)=FLANGE BRACE AT ONE SIDE ONLY
FBXXA(2)=FLANGE BRACE AT BOTH SIDES
A - L2x2x14

2ND FLOOR TO 3RD FLOOR

SPLICE PLATE & BOLT TABLE

Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	1.000	3.00	8"	3/4"	1'-10 9/16"
SP-2	4	4	0	A325	0.750	2.50	6"	5/8"	1'-10 5/16"

STIFFENER TABLE

Mark	Stiff Mark	Plate Size Width	Thick	Length
RF1-1	St- 1	3.750	0.250	12.00
RF1-3	St- 1	2.500	0.250	17.50

CAP PLATE BOLTS

Mark	Qty	Type	Dia	Length
RF1-5	4	A325	0.750	2.00

BASE PLATE TABLE

Col Mark	Plate Size Width	Thick	Length
BP-1	8"	1/2"	1'-2"
BP-2	1'-2"	5/8"	1'-2"

FBXXA(1)=FLANGE BRACE AT ONE SIDE ONLY
FBXXA(2)=FLANGE BRACE AT BOTH SIDES
A - L2x2x14

3RD FLOOR TO ROOF

SPLICE PLATE & BOLT TABLE

Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	0.625	2.25	6"	1/2"	1'-7 1/4"
SP-2	4	4	0	A325	0.625	2.00	6"	1/2"	1'-7 1/4"

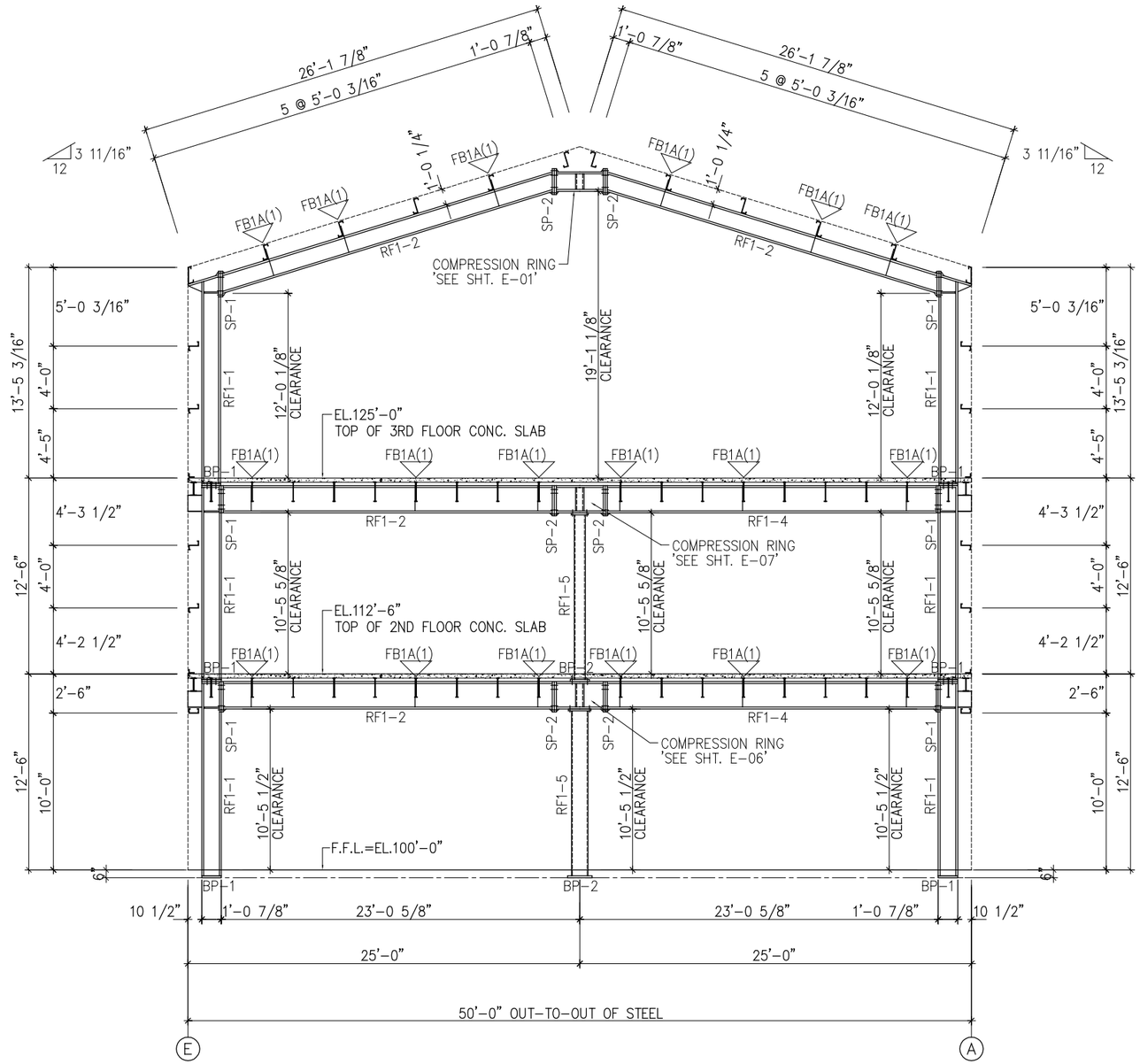
STIFFENER TABLE

Mark	Stiff Mark	Plate Size Width	Thick	Length
RF1-1	St- 1	2.410	0.313	10.96

BASE PLATE TABLE

Col Mark	Plate Size Width	Thick	Length
BP-1	8"	1/2"	1'-1"

FBXXA(1)=FLANGE BRACE AT ONE SIDE ONLY
FBXXA(2)=FLANGE BRACE AT BOTH SIDES
A - L2x2x14



RIGID FRAME ELEVATION: FRAME LINE 1 2

1ST FLOOR TO 2ND FLOOR

MEMBER TABLE

Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	12.0/12.0	0.188	127.4	8 x 3/8" x 149.1	8 x 1/2" x 127.4
	12.0/12.0	0.375	21.7	8 x 3/8" x 22.9	
RF1-2	18.0/18.0	0.188	240.0	6 x 3/8" x 257.0	6 x 3/8" x 257.0
	18.0/18.0	0.188	17.0		
RF1-3	18.0/18.0	0.188	17.4	6 x 3/8" x 17.4	6 x 3/8" x 34.8
	18.0/18.0	0.188	17.4	6 x 3/8" x 17.4	
RF1-4	18.0/18.0	0.188	240.0	6 x 3/8" x 257.0	6 x 3/8" x 257.0
	18.0/18.0	0.188	17.0		
RF1-5	D127x250				

2ND FLOOR TO 3RD FLOOR

MEMBER TABLE

Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	12.0/12.0	0.188	129.7	6 x 1/2" x 149.0	6 x 1/2" x 129.7
	12.0/12.0	0.250	19.3	6 x 1/2" x 23.0	
RF1-2	18.0/18.0	0.188	240.0	6 x 5/16" x 256.9	6 x 5/16" x 256.9
	18.0/18.0	0.188	16.9		
RF1-3	18.0/18.0	0.188	17.4	6 x 5/16" x 17.4	6 x 5/16" x 34.8
	18.0/18.0	0.188	17.4	6 x 5/16" x 17.4	
RF1-4	18.0/18.0	0.188	240.0	6 x 5/16" x 256.9	6 x 5/16" x 256.9
	18.0/18.0	0.188	16.9		
RF1-5	D862x322				

3RD FLOOR TO ROOF

MEMBER SIZE TABLE

MARK	MEMBER	LENGTH
RF1-1	W12x14	12'-8 5/8"
RF1-2	W12x14	24'-1 15/16"

FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
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RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

DESCRIPTION	RIGID FRAME ELEVATION		
CUSTOMER	JEFF SHEPARD		
END USER	JEFFREY SHEPARD		
END USE	RESIDENTIAL	BUILDING	A
STREET	1684 STONEY CREEK RD		
CITY ST ZIP	BEDFORD, VA 24523		

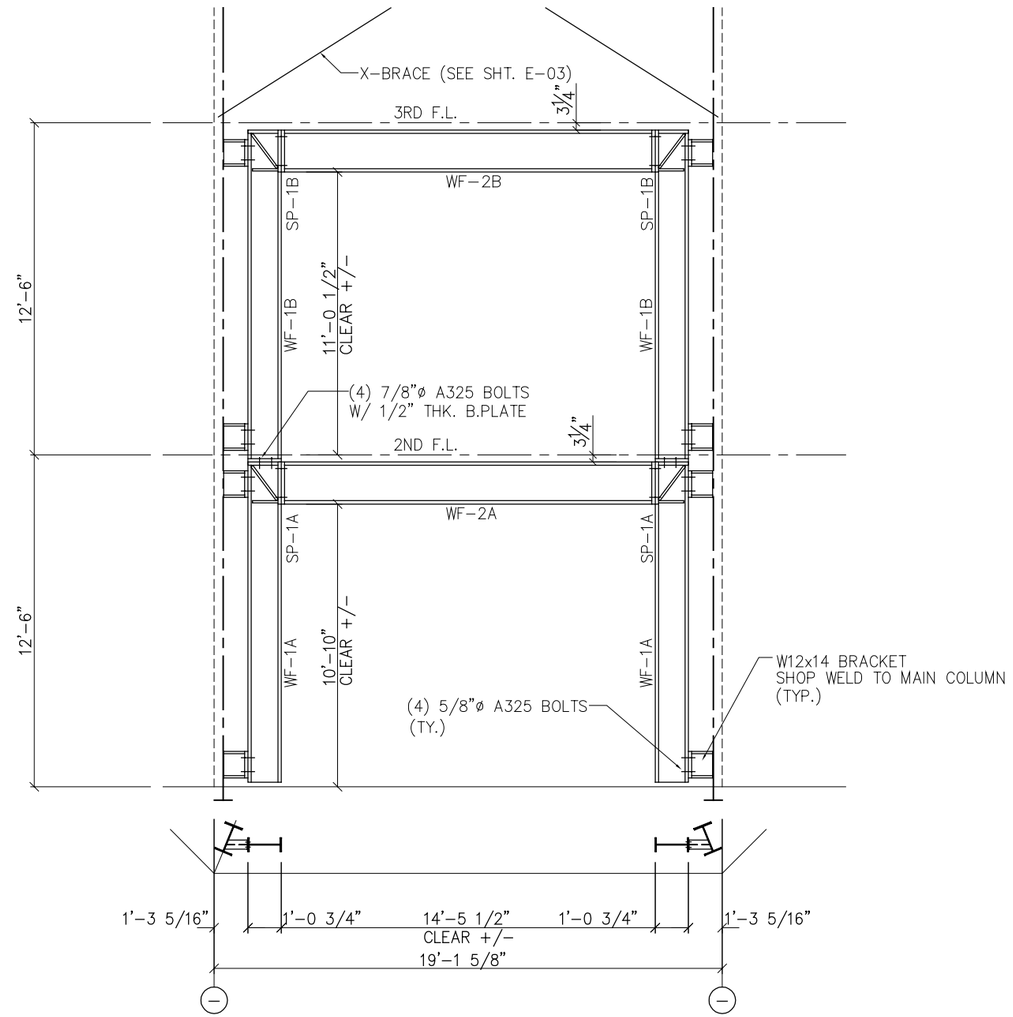
SALES NO.:	JOB NO.:	SCALE:	DWG. NO.:	ISSUE:
68375	147664	N.T.S.	E04 OF 11	A

SPLICE BOLTS					
Splice Mark	Quan Top/Bot	Bolt Type	Bolt Dia	Bolt Length	
SP– 1A	2	2	A325	1.000	3.00

SPLICE BOLTS					
Splice Mark	Quan Top/Bot	Bolt Type	Bolt Dia	Bolt Length	
SP– 1B	2	2	A325	0.875	2.75

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF–2A	W16642	14'–4 3/4"
WF–1A	W12662	12'–6 1/2"

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF–2B	W16642	14'–4 3/4"
WF–1B	W12662	12'–8 1/2"



WIND BENT ELEVATION

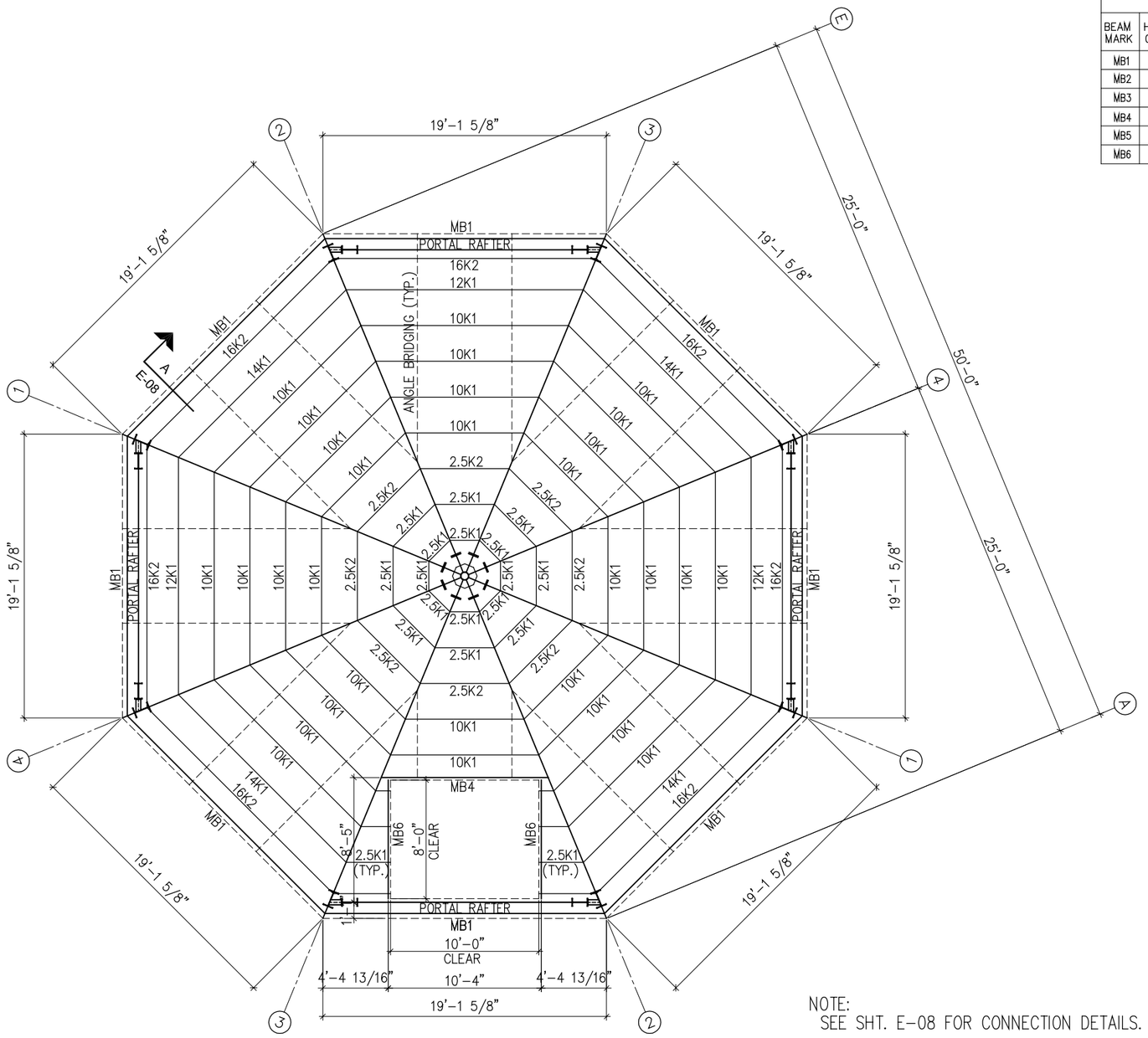
FOR PERMIT

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT RIGID GLOBAL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY RIGID IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN RIGID ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

DESCRIPTION	WIND BENT ELEVATION			
CUSTOMER	JEFF SHEPARD			
END USER	JEFFREY SHEPARD			
END USE	RESIDENTIAL	BUILDING	A	
STREET	1684 STONEY CREEK RD			
CITY ST ZIP	BEDFORD, VA 24523			
SALES NO.: 68375	JOB NO.: 147664	SCALE: N.T.S.	DWG. NO.: E05 OF 11	ISSUE: A



NOTE:
SEE SHT. E-08 FOR CONNECTION DETAILS.

BEAM SCHEDULE					
BEAM MARK	HOT-ROLLED OR BUILT-UP	ANGLE CLIP CONNECTION	SHEAR PLATE CONNECTION		
			NO.	BOLT SIZE	END PLATE
MB1	W10x12	BC-2	2	3/4" ø	3/8" THK.
MB2	W10x12	BC-2	2	3/4" ø	3/8" THK.
MB3	W12x14	BC-2	2	3/4" ø	3/8" THK.
MB4	W12x14	BC-2	2	3/4" ø	3/8" THK.
MB5	W12x14	BC-2	2	3/4" ø	3/8" THK.
MB6	W12x14	BC-2	2	3/4" ø	3/8" THK.

NOTES:

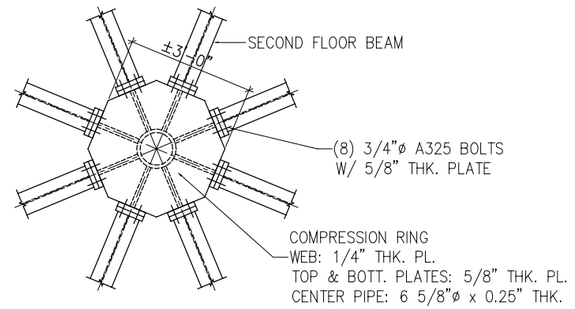
1. JOISTS SHALL BE FIELD WELDED TO BEAMS OR SEAT PLATES UNLESS NOTED AS BOLTED. FOR BOLTED JOISTS, USE 1/2 X 1-1/4 A-307 BOLT.
2. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOISTS. THE ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED THERETO. HORIZONTAL OR DIAGONAL BRIDGING SHALL BE ATTACHED TO JOISTS BY MEANS OF WELDING OR MECHANICAL MEANS.
3. EDGE ANGLES SHALL BE TACK WELDED TO BEAMS OR JOISTS.
4. MEZZANINE DECK SHALL BE FASTENED WITH #12-24 X 1-1/4 HEX WASHER HEAD (TCP5 ULTRA-Z POINT) SCREWS @ 12" O.C.
   SIDE LAP SCREW SHALL BE 10-16x1" T-2 @ 36" O.C. MAXIMUM.

FIELD WELDING NOTES:

1. WELDING ELECTRODES MUST BE "E7018". THIS DENOTES A SPECIFIC TYPE OF ELECTRODE WITH A 70 KSI YIELD STRENGTH. THIS ELECTRODE IS COMPATIBLE WITH STEEL SUPPLIED BY RIGID BUILDING SYSTEMS, AND IS ACCEPTABLE FOR ALL WELDING POSITIONS.
2. WELDING MUST BE PERFORMED BY A CERTIFIED WELDER WHO IS CERTIFIED IN THE APPROPRIATE POSITION, AND CONFORM TO THE REQUIREMENTS OF THE AWS STRUCTURAL WELDING CODE D1.1, LATEST EDITION.
3. PRE-HEAT THE JOINT AS NEEDED TO ELIMINATE MOISTURE CONTAMINATION AND CRACKING. REFER TO THE AWS STRUCTURAL WELDING CODE D1.1, LATEST EDITION. ANY SHOP PAINT ON SURFACES ADJACENT TO JOINTS TO BE FIELD WELDED SHALL BE WIRE BRUSHED TO REDUCE THE PAINT FILM TO A MINIMUM. SLAG, RUST, GREASE, AND OTHER HARMFUL FOREIGN MATERIAL MUST BE REMOVED.

IMPORTANT NOTE:

RIGID GLOBAL BUILDINGS IS NOT RESPONSIBLE FOR THE DESIGN AND THE THICKNESS OF MEZZANINE CONCRETE SLAB AND ITS REINFORCEMENTS. ONLY MEZZANINE COMPONENTS CALLED OUT IN THE CONTRACT WILL BE SUPPLIED BY 'RIGID'.



FOR PERMIT

SECOND FLOOR FRAMING PLAN

DL = 50 PSF (15.5 PSF FLOOR HEATING SYSTEM INCLUSIVE)
LL = 100 PSF

DECKING:

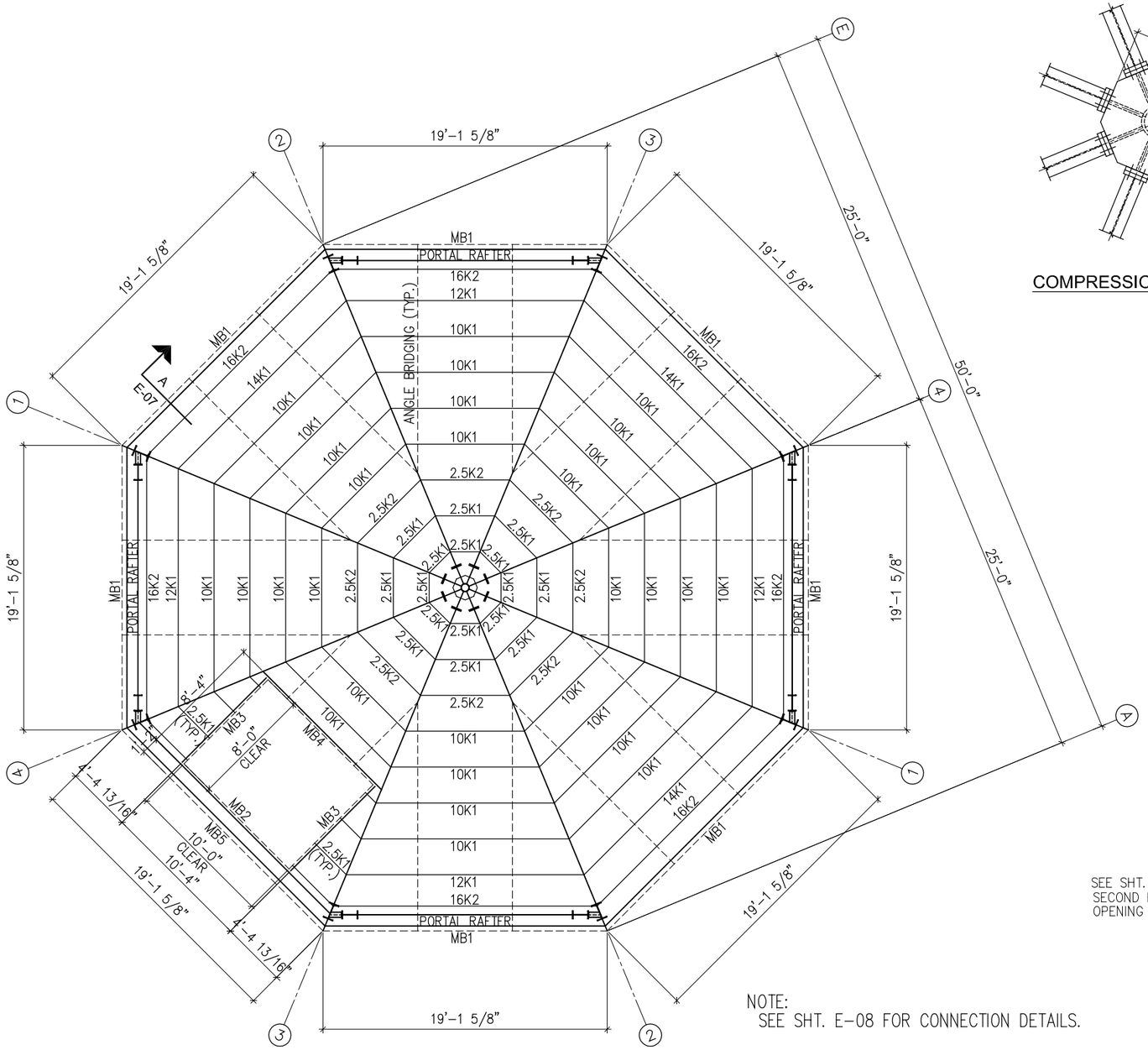
3.25" THK. LIGHT WEIGHT CONC. ON 22 GA. 1.5B G-60 GALV. DECKING
TOP OF FLOOR ELEVATION = 12'-6"
CLEAR HT. UNDER BEAM = 10'-5 5/8"
JOIST SPACING: 2'-6" MAX.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

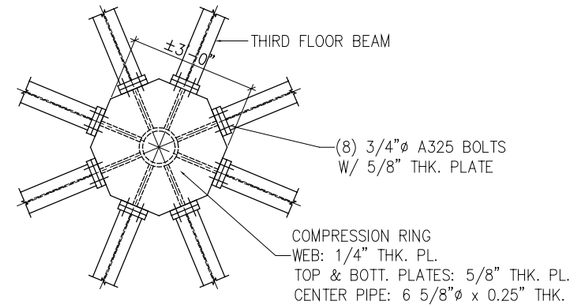
RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

DESCRIPTION	SECOND FLOOR FRAMING		
CUSTOMER	JEFF SHEPARD		
END USER	JEFFREY SHEPARD		
END USE	RESIDENTIAL	BUILDING	A
STREET	1684 STONEY CREEK RD		
CITY ST ZIP	BEDFORD, VA 24523		

SALES NO.	JOB NO.	SCALE	DWG. NO.	ISSUE
68375	147664	N.T.S.	E06 OF 11	A

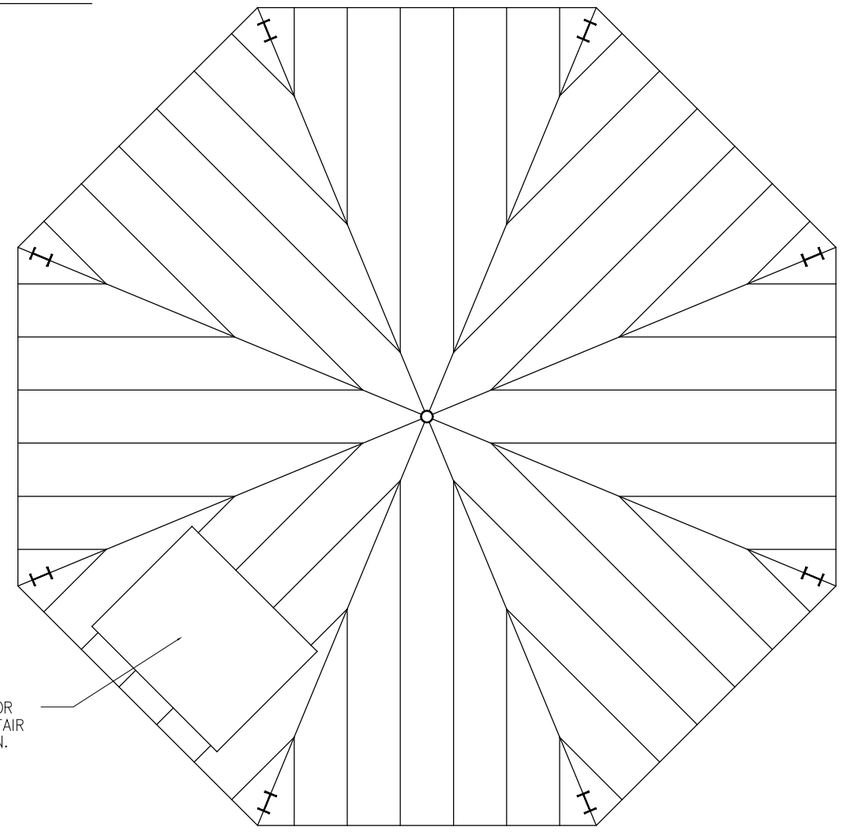


NOTE:
SEE SHT. E-08 FOR CONNECTION DETAILS.



BEAM SCHEDULE					
BEAM MARK	HOT-ROLLED OR BUILT-UP	ANGLE CLIP CONNECTION	SHEAR PLATE CONNECTION		
			NO.	BOLT SIZE	END PLATE
MB1	W10x12	BC-2	2	3/4" ø	3/8" THK.
MB2	W10x12	BC-2	2	3/4" ø	3/8" THK.
MB3	W12x14	BC-2	2	3/4" ø	3/8" THK.
MB4	W12x14	BC-2	2	3/4" ø	3/8" THK.
MB5	W12x14	BC-2	2	3/4" ø	3/8" THK.
MB6	W12x14	BC-2	2	3/4" ø	3/8" THK.

COMPRESSION RING DETAIL



SEE SHT. E-06 FOR SECOND FLOOR STAIR OPENING LOCATION.

DECKING LAY-OUT (2ND & THIRD FLOOR)

PANELS: 22Ga. 1.5B DECK (GALV'Z G-60)

THIRD FLOOR FRAMING PLAN

DL = 50 PSF (15.5 PSF FLOOR HEATING SYSTEM INCLUSIVE)
LL = 100 PSF

DECKING:

3.25" THK. LIGHT WEIGHT CONC. ON 22 GA. 1.5B G-60 GALV. DECKING
TOP OF FLOOR ELEVATION = 25'-0" (FROM F.F.L.)
CLEAR HT. UNDER BEAM = 10'-5 1/4"
JOIST SPACING: 2'-6" MAX.

FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
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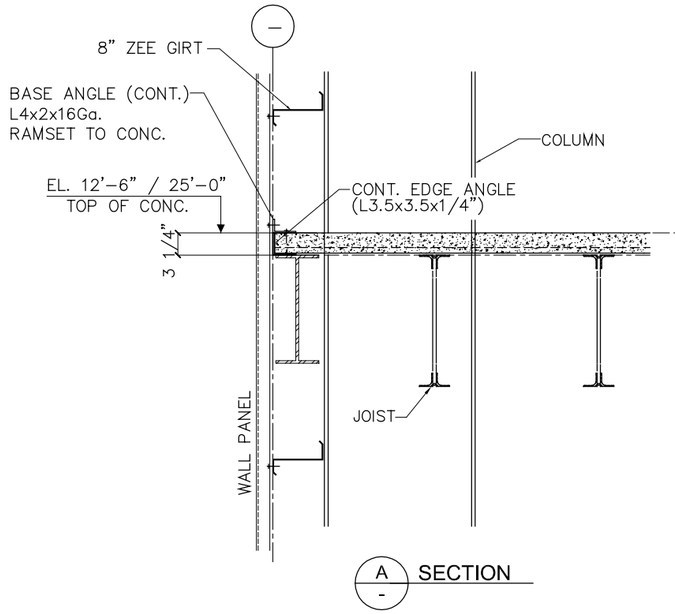
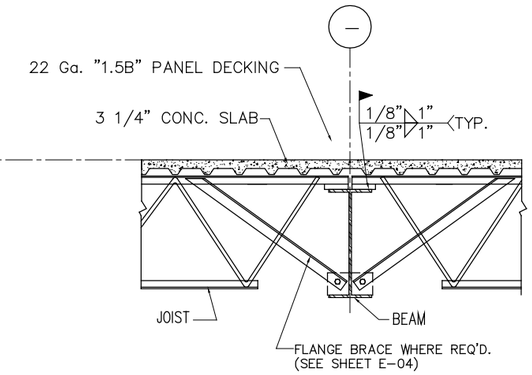
RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

DESCRIPTION	THIRD FLOOR FRAMING PLAN		
CUSTOMER	JEFF SHEPARD		
END USER	JEFFREY SHEPARD		
END USE	RESIDENTIAL	BUILDING	A
STREET	1684 STONEY CREEK RD		
CITY ST ZIP	BEDFORD, VA 24523		

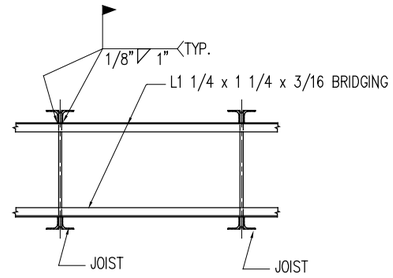
SALES NO.	JOB NO.	SCALE:	DWG. NO.	ISSUE:
68375	147664	N.T.S.	E07 OF 11	A



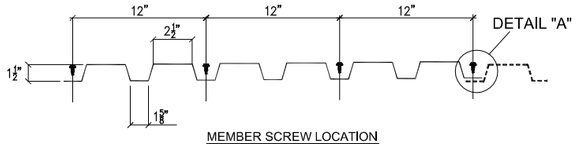
A SECTION



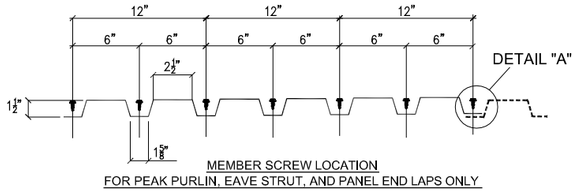
JOIST TO BEAM CONN.



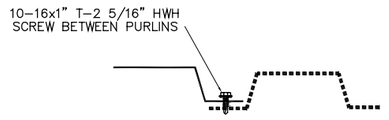
BRIDGING CONNECTION DETAIL



MEMBER SCREW LOCATION
FOR ALL MEMBERS UNLESS NOTED OTHERWISE

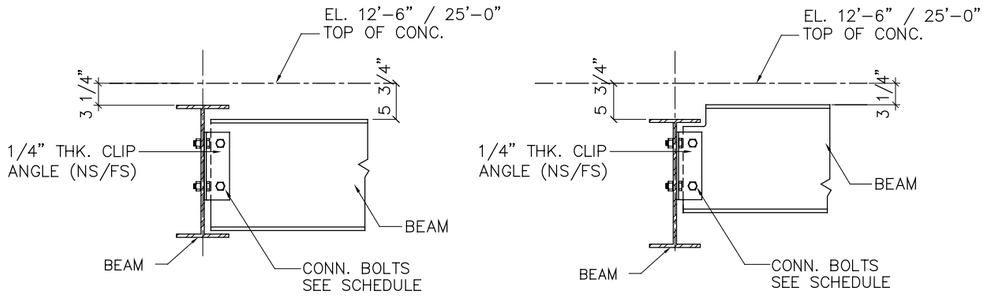


MEMBER SCREW LOCATION
FOR PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS ONLY



DETAIL "A"
TYP. SIDE LAP DETAIL

"1.5B" DECKING PANEL SCREW LAYOUT



BEAM TO BEAM CONNECTION

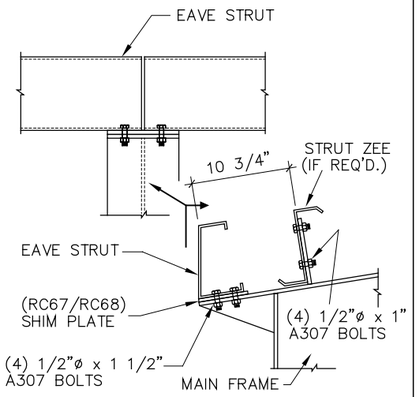
FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

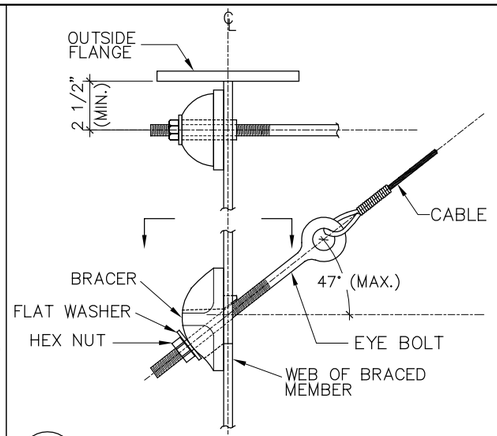
RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

DESCRIPTION	FLOOR FRAMING DETAILS		
CUSTOMER	JEFF SHEPARD		
END USER	JEFFREY SHEPARD		
END USE	RESIDENTIAL	BUILDING	A
STREET	1684 STONEY CREEK RD		
CITY ST ZIP	BEDFORD, VA 24523		

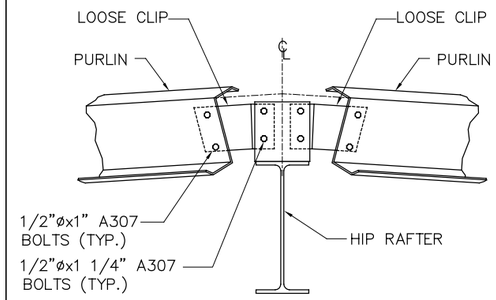
SALES NO.	JOB NO.	SCALE	DWG. NO.	ISSUE
68375	147664	N.T.S.	E08 OF 11	A



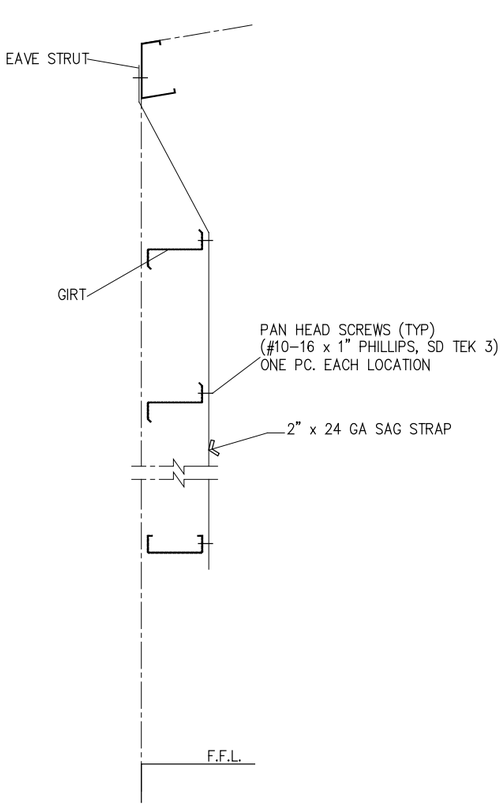
J24 LOW EAVE DETAIL (BY-PASS CONDITION)
AT END FRAME



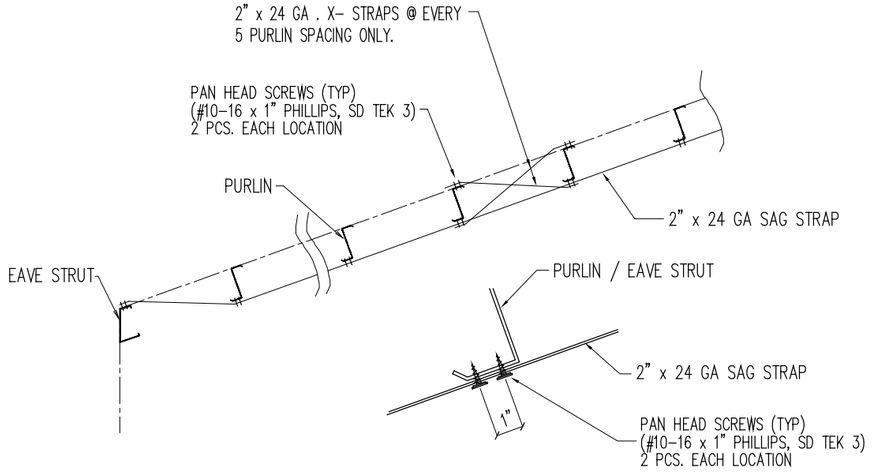
Q2 CABLE BRACE CONNECTION DETAIL



1 SF41 PURLIN TO HIP RAFTER CONNECTION



TYP. WALL SAG STRAP DETAIL



TYPICAL PURLIN SAG STRAP CONNECTION DETAIL

FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

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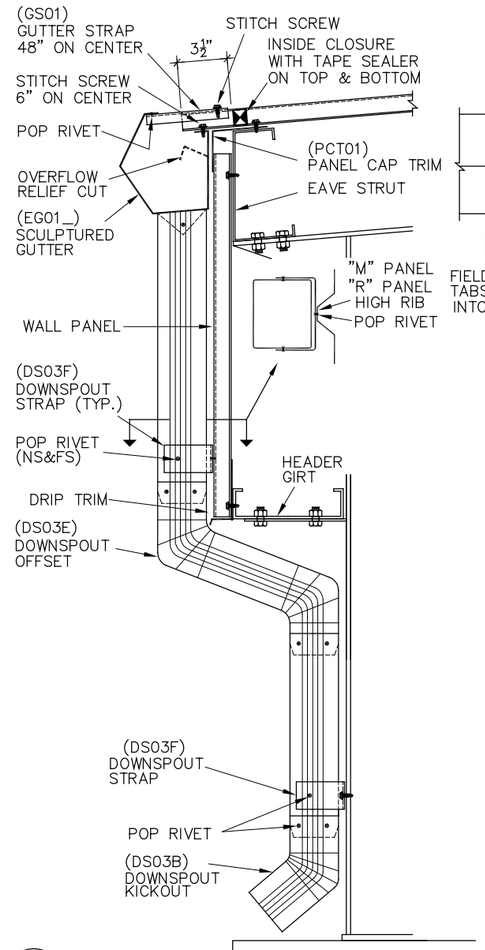
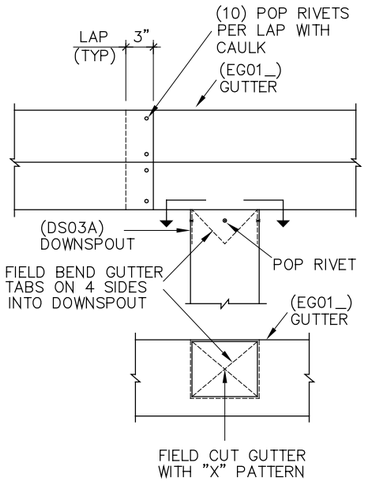
Fax : (281) 443-9064

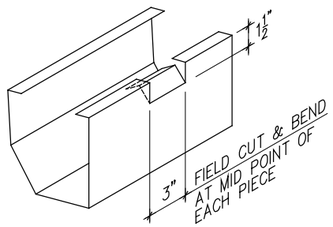
DESCRIPTION	DETAIL DRAWINGS				
CUSTOMER	JEFF SHEPARD				
END USER	JEFFREY SHEPARD				
END USE	RESIDENTIAL	BUILDING	A		
STREET	1684 STONEY CREEK RD				
CITY ST ZIP	BEDFORD, VA 24523				
SALES NO.: 68375	JOB NO.: 147664	SCALE: N.T.S.	DWG. NO.: E09 OF 11	ISSUE: A	



1 ST22 DOWNSPOUT & GUTTER DETAIL
WITH PARTIALLY OPEN WALL

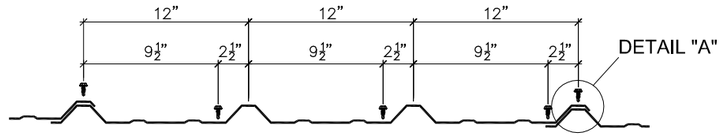


DOWNSPOUT TO GUTTER DETAIL

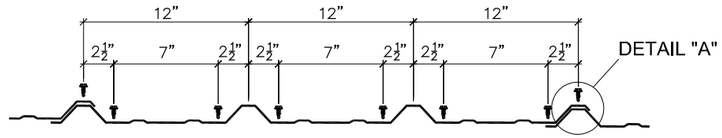


GUTTER OVERFLOW RELIEF

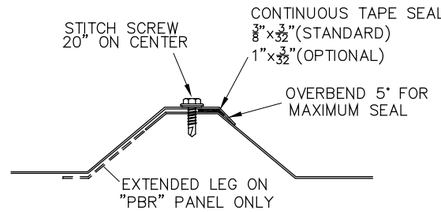
D.S. STRAP QUANTITY	
E.H.	QTY.
10'–0	2
12'–0	2
14'–0	2
16'–0	2
20'–0	2
25'–0	3



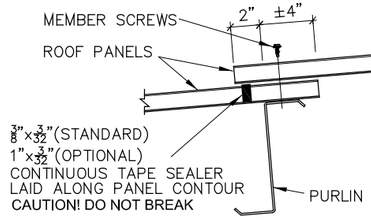
MEMBER SCREW LOCATION
FOR ALL MEMBERS UNLESS NOTED OTHERWISE



MEMBER SCREW LOCATION
FOR PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS ONLY



DETAIL "A"
TYP. SIDE LAP DETAIL



DETAIL "B"
TYP. PANEL END LAP DETAIL

1 ST01 "R" and "PBR" ROOF PANEL SCREW LAYOUT

NOTE!

SCREW PATTERNS SHOWN, SATISFY UL-90 REQUIREMENT FOR ROOF.

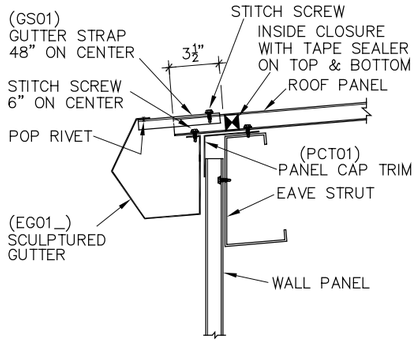
FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

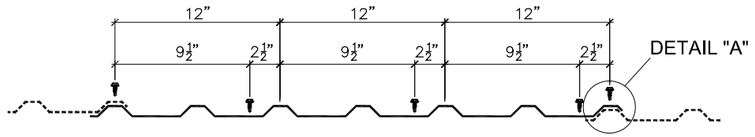
RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone: (281) 443-9065
Fax: (281) 443-9064

DESCRIPTION	DETAIL DRAWINGS		
CUSTOMER	JEFF SHEPARD		
END USER	JEFFREY SHEPARD		
END USE	RESIDENTIAL	BUILDING	A
STREET	1684 STONEY CREEK RD		
CITY ST ZIP	BEDFORD, VA 24523		

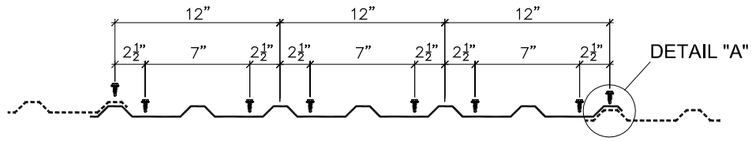
SALES NO.	JOB NO.	SCALE:	DWG. NO.	ISSUE:
68375	147664	N.T.S.	E10 OF 11	A



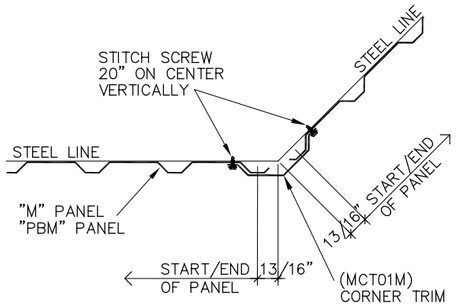
1 ST46 LOW EAVE DETAIL WITH SCULPTURED GUTTER SHEETED WALL



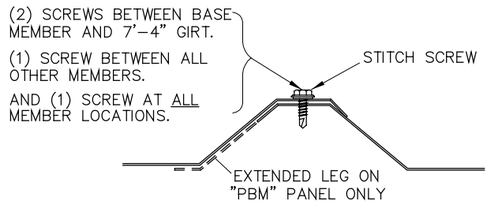
MEMBER SCREW LOCATION
FOR ALL MEMBERS UNLESS NOTED OTHERWISE



MEMBER SCREW LOCATION
FOR BASE, EAVE, RAKE, AND PANEL END LAPS ONLY

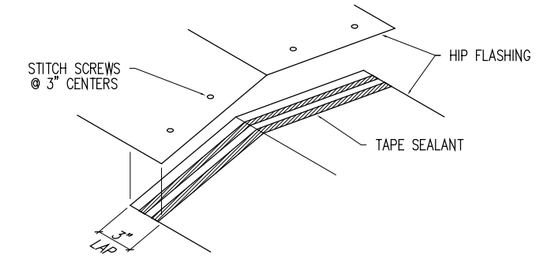


2 ST83 OUTSIDE CORNER DETAIL OFF MODULE

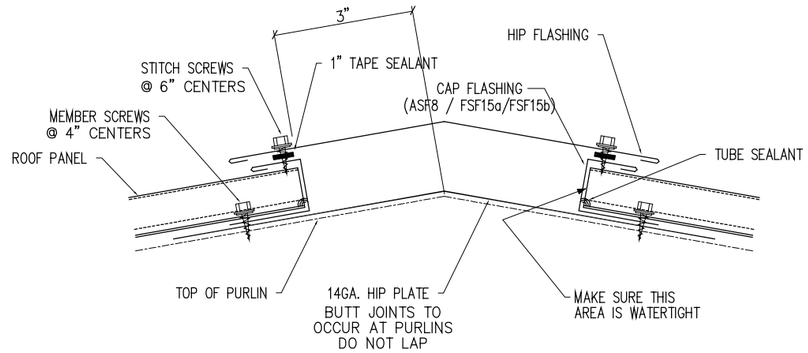


DETAIL "A"
TYP. SIDE LAP DETAIL

1 ST04 "M" AND "PBM" WALL PANEL SCREW LAYOUT



DETAIL AT HIP TRIM SPLICE



SECTION THRU HIP

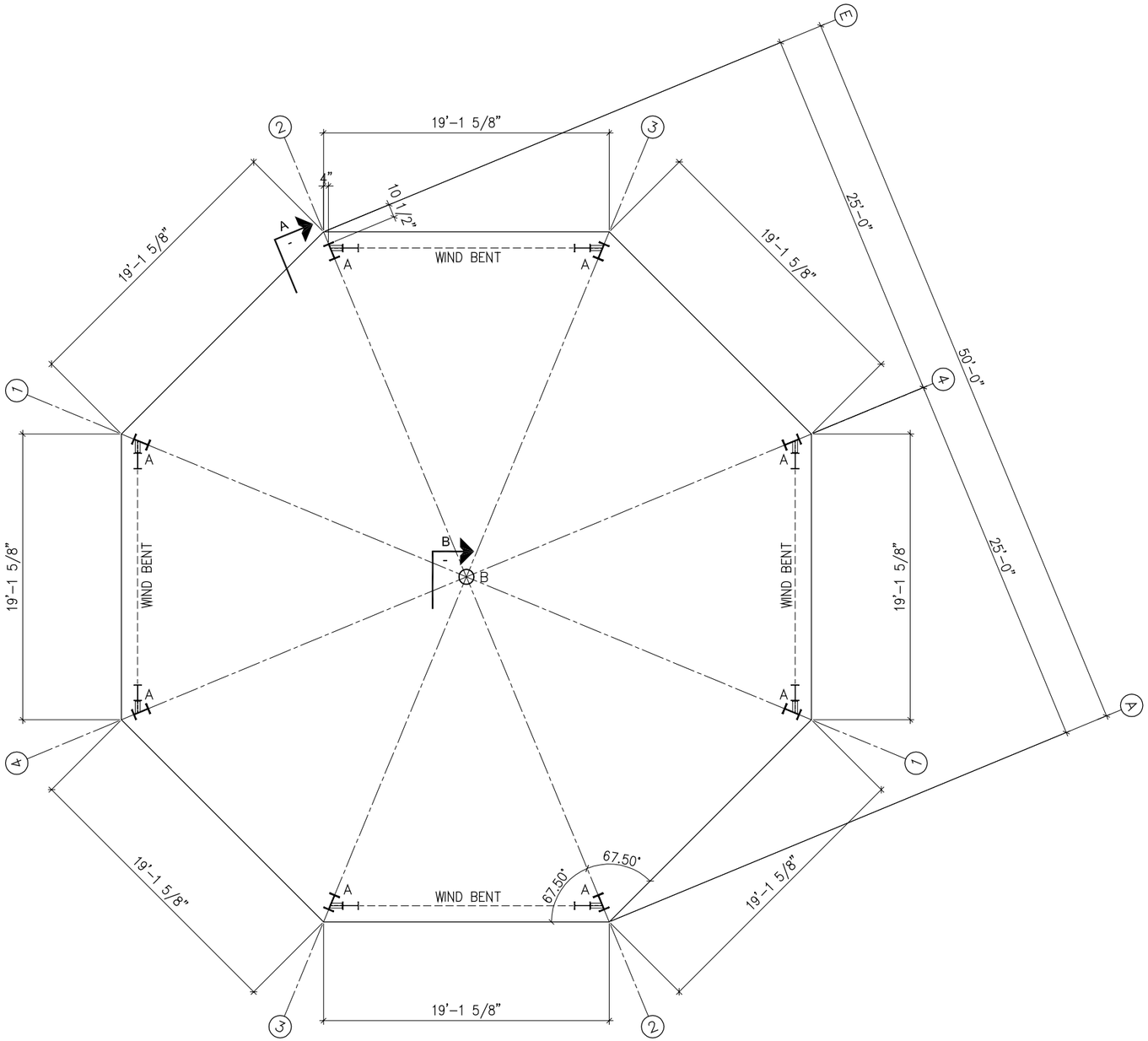
FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

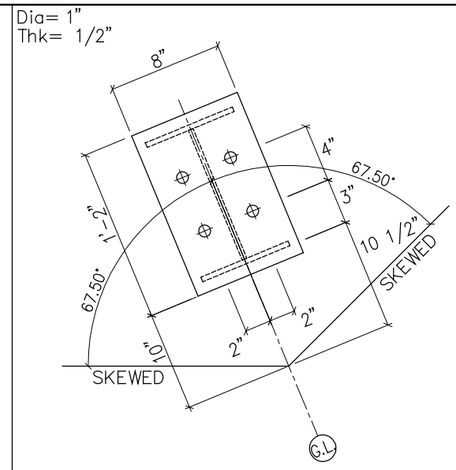
DESCRIPTION	DETAIL DRAWINGS		
CUSTOMER	JEFF SHEPARD		
END USER	JEFFREY SHEPARD		
END USE	RESIDENTIAL	BUILDING	A
STREET	1684 STONEY CREEK RD		
CITY ST ZIP	BEDFORD, VA 24523		

SALES NO.:	JOB NO.:	SCALE:	DWG. NO.:	ISSUE:
68375	147664	N.T.S.	E11 OF 11	A

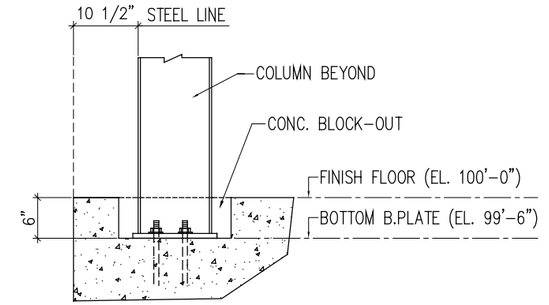


COLUMN LAYOUT PLAN

NOTE: All Base Plates @ 99'-6" (U.N.)



DETAIL A



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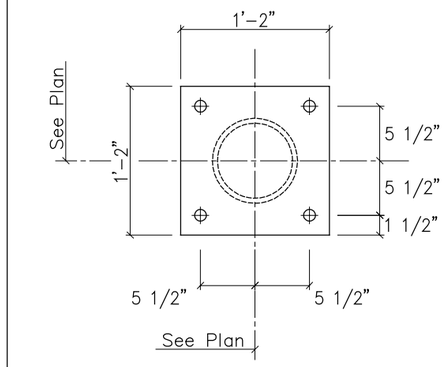
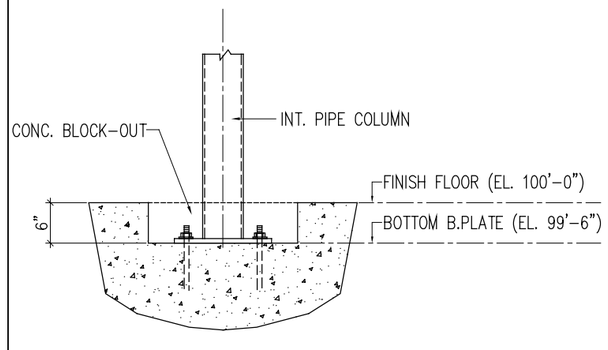


DETAIL B



SECTION "B"

FOR PERMIT

NOTE:

ONLY ANCHOR BOLTS SETTING PLAN ISSUED & STAMPED "FOR CONSTRUCTION" SHALL BE USED IN SETTING ANCHOR BOLTS. 'RIGID GLOBAL BUILDINGS' SHALL NOT BE RESPONSIBLE FOR ERROR OR DISCREPANCY IF THE DRAWING USED IS NOT VALID FOR CONSTRUCTION.

QTY.	SYMBOL	DIA.	PROJ.
–	✦	1/2"	1"
–	⌖	5/8"	2"
4	⌖	3/4"	2 1/2"
–	⌖	7/8"	2 3/4"
32	⌖	1"	3"
–	⌖	1 1/8"	3 1/2"
–	⌖	1 1/2"	3 1/2"

ANCHOR BOLT DETAIL

ANCHOR BOLT PROJECTION "PROJ." IS MEASURED FROM BOTTOM OF BASE PLATE

LENGHT OF "PROJ." SHOWN IS FOR ONE NUT + ONE WASHER

DETAIL OF ANCHOR BOLT AS PER THE SUPPLIER

NUTS & WASHERS BY SUPPLIER

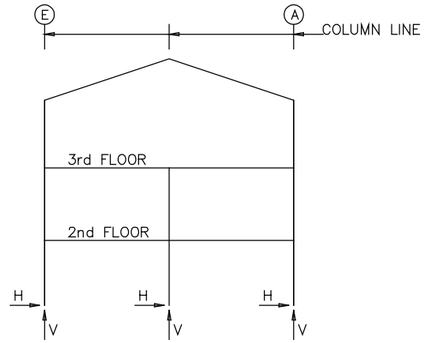
ANCHOR BOLTS NOT BY RIGID GLOBAL BUILDINGS

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	APPROVAL/PERMIT	10/16/20	JGM	RCR	JDB

RIGID GLOBAL BUILDINGS
18933 Aldine Westfield
Houston, Tx. 77073
Phone : (281) 443-9065
Fax : (281) 443-9064

DESCRIPTION	COLUMN LAYOUT PLAN			
CUSTOMER	JEFF SHEPARD			
END USER	JEFFREY SHEPARD			
END USE	RESIDENTIAL	BUILDING	A	
STREET	1684 STONEY CREEK RD			
CITY ST ZIP	BEDFORD, VA 24523			
SALES NO.: 68375	JOB NO.: 147664	SCALE: N.T.S.	DWG. NO.: F01 OF 2	ISSUE: A

FRAME LINES: 1 2



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Bottom B.Plate (in)
1*	E	3	11.3	25.4	6	−9.3	2.1	4	1.000	8.000	14.00	0.500	−6.0
		5	10.5	42.2	9	1.3	−2.5						
1*	A	7	9.3	2.1	2	−11.3	25.5	4	1.000	8.000	14.00	0.500	−6.0
		4	−10.5	42.2	8	−1.3	−2.5						
1*	@25.0	1	0.0	198.1				4	0.750	14.00	14.00	0.625	−6.0

1* Frame lines: 1 2 3 4

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data.

Width (ft)	:	46.19
Length (ft)	:	19.13
Eave Height (ft)	:	38 / 38
Roof Slope (rise/12)	:	4.0:12 / 4.0:12
Design Code	:	IBC 15
Enclosure	:	Closed
Dead Load (psf)	:	2.000
Collateral Load (psf)	:	5
Wind Speed (mph)	:	115 mph
Wind Importance Factor	:	1.00
Wind Exposure	:	C
Live Load (psf)	:	20.00
Frame Live Load (psf)	:	20
Ground Snow Load (psf)	:	40.000
Roof Snow Load (psf)	:	30
Snow Exposure	:	1.000
Snow Importance Factor	:	1.000
Thermal Factor	:	1.000
Seismic Importance Factor	:	1.00
Spectral Response Accel.	:	Ss=0.170 : S1=0.072
Spectral Response Coeff.	:	Sds=0.181 : Sd1=0.115
Seismic Coeff. (Fa*Ss)	:	0.272 : Fa=1.600
Seismic Design Category	:	B

5. Loading conditions are:
   1 Dead+Collateral+Floor_Live
   2 Dead+0.6Wind_Left1
   3 Dead+0.6Wind_Right1
   4 Dead+Collateral+0.75Snow+0.45Wind_Left1+0.75Slide_Snow+0.75Floor_Live
   5 Dead+Collateral+0.75Snow+0.45Wind_Right1+0.75Slide_Snow+0.75Floor_Live
   6 0.6Dead+0.6Wind_Left2
   7 0.6Dead+0.6Wind_Right2
   8 0.6Dead+0.6Wind_Long1L
   9 0.6Dead+0.6Wind_Long2L

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	1								(h)
F_SW	A	3,2 1,4	9.2	39.1	3.5	14.6			(d)
R_EW	2								(h)
B_SW	E	3,2 1,4	9.2	39.1	3.5	14.6			(d)

(d)X−Bracing above wind bent, base above finish floor
(h)Rigid frame at endwall

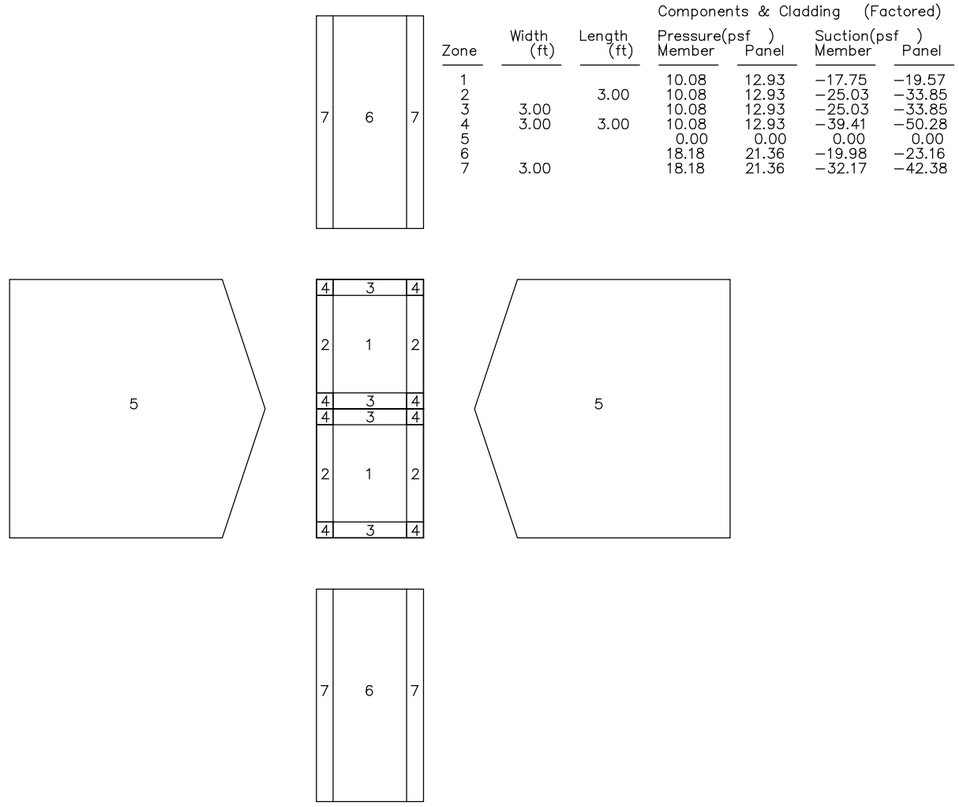
RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Floor Horiz	Floor Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert
1*	E	1.4	20.8	0.0	1.6	0.0	4.2	2.3	15.4	0.0	6.2	−16.0	−19.6
1*	A	−1.4	20.8	0.0	1.6	0.0	4.2	−2.3	15.4	0.0	6.2	−16.6	7.8
1*	@25.0	0.0	78.5	0.0	0.0	0.0	0.0	0.0	119.6	0.0	0.0	0.0	0.0

Frame Line	Column Line	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert
1*	E	16.6	7.7	−16.9	−17.3	15.7	5.5	2.7	−23.9	0.9	−25.0	−2.3	−1.1
1*	A	16.0	−19.6	−15.7	5.5	16.9	−17.3	−0.9	−25.0	−2.7	−23.9	−2.3	1.1
1*	@25.0	0.0	0.0	0.0	−0.1	0.0	−0.1	0.0	0.3	0.0	0.3	0.0	0.0

Frame Line	Column Line	Seismic_Right Horiz	Seismic_Right Vert	Seismic_Long Horiz	Seismic_Long Vert	F1PAT_FL_1 Horiz	F1PAT_FL_1 Vert	F1PAT_FL_2 Horiz	F1PAT_FL_2 Vert	F1PAT_FL_3 Horiz	F1PAT_FL_3 Vert	F1PAT_FL_4 Horiz	F1PAT_FL_4 Vert
1*	E	2.3	1.1	0.0	−5.3	0.1	4.2	0.1	0.2	1.0	15.6	1.0	−0.4
1*	A	2.3	−1.1	0.0	−5.3	−0.1	0.2	−0.1	4.2	−1.0	−0.4	−1.0	15.6
1*	@25.0	0.0	0.0	0.0	0.0	0.0	−4.4	0.0	−4.4	0.0	7.3	0.0	7.3

1* Frame lines: 1 2 3 4



Components & Cladding (Factored)

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			10.08	12.93	−17.75	−19.57
2		3.00	10.08	12.93	−25.03	−33.85
3	3.00		10.08	12.93	−25.03	−33.85
4	3.00	3.00	10.08	12.93	−39.41	−50.28
5			0.00	0.00	0.00	0.00
6			18.18	21.36	−19.98	−23.16
7	3.00		18.18	21.36	−32.17	−42.38

Design Calculation Wind

FOR PERMIT

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
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CITY ST ZIP	BEDFORD, VA 24523		

SALES NO.	JOB NO.	SCALE	DWG. NO.	ISSUE
68375	147664	N.T.S.	F02 OF 2	A